Excavation of the HK Cavern

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Contents

- Summary of baseline design
 - Geological survey results
 - Cavity stability study, support design
 - Scheduling excavation process
- Ongoing study

Hyper-K candidate site



- ♦ 8km south from Super-K
 - same T2K beam off-axis angle (2.5 degree)
 - same baseline length (295km)
- 2.6km horizontal drive from entrance



Side view of the site



geological survey and rock property measurements

- The candidate site is dominated by gneiss and Inishi-type rock. Contaminated with skarn, aplite, clay.
- Survey in the tunnels and the boreholes were performed
 - Obtain classification of rock mass
- in-situ rock stress measurements were performed at -300mL.
- Borehole loading tests were also performed to estimate the mechanical properties of in-situ rock mass.



Rock class model (-370mL, tank floor level)



| | Cavern | ana | lysis |
|--|--------|-----|-------|
|--|--------|-----|-------|

| | | Rock mass class (%) | | | | | | | | | | | | |
|------------|-----|---------------------|---------|------|------|-----|--|--|--|--|--|--|--|--|
| | А | В | СН | СМ | CL | D | | | | | | | | |
| North-side | 0.0 | 0.0 | 71.8 | 28.2 | 0.0 | 0.0 | | | | | | | | |
| Cavern | | 71.8 | | | 28.2 | | | | | | | | | |
| South-side | 0.0 | 9.0 | 70.7 | 20.3 | 0.0 | 0.0 | | | | | | | | |
| Cavern | | 79.7 | | | 20.3 | | | | | | | | | |
| Tatal | 0.0 | 4.5 | 71.3 | 24.2 | 0.0 | 0.0 | | | | | | | | |
| | | 75.8 | | 24.2 | | | | | | | | | | |
| | · | | Plane V | iew | | | | | | | | | | |

Table 3.4 Input Property Values

| Rock mass class | В | СН | $\mathbf{C}\mathbf{M}$ |
|--------------------------------------|-------|-------|------------------------|
| Young's modulus(kN/mm ²) | 10.10 | 3.43 | 1.22 |
| Poisson's ratio | 0.25 | 0.25 | 0.25 |
| Cohesion(N/mm ²) | 4.90 | 2.40 | 1.40 |
| Internal friction angle(deg) | 60.00 | 50.00 | 45.00 |

Cross-Section View



Longitudinal Sectional View







- ~I 2m depth of loosened region is affordable by appropriate design of PS anchors and rock bolts.
- Past experiences in underground power plants in Japan.

anchors and cable-bolts



Excavation (bench cut)

Cross section View



Side View







Waste rock disposal place



• need to secure the disposal place, for example, geological survey, stability study are yet to be done.

| | 1stYear | | | | | 1stYear 2nd Year | | | | | | | 3rd Year | | | | | | | | | 4th Year | | | | | | | 5th Year | | | | | | | | | | | | | | | | | |
|---|---------|---|---|----|------|------------------|---|-----|---|-----|-----|---|------------|------|------|-----|---|---|-----|-----|---|----------|------|------|----|-----|-----|---|----------|-----|-------|------|-----|------|-------|----|---|-----|-----|-------|------|------|----------|----|---|-----|
| | 4 5 | 6 | 7 | 89 | 10 1 | 11 12 | 1 | 2 3 | 4 | 5 6 | 6 7 | 8 | 9 | 10 1 | 1 12 | 2 1 | 2 | 3 | 4 5 | 5 6 | 7 | 8 9 | 9 10 |) 11 | 12 | 1 2 | 2 3 | 4 | 5 | 6 | 7 8 | 3 9 | 10 | 11 1 | 2 1 | 2 | 3 | 4 ! | 56 | 6 7 | 8 | 9 1 | 0 11 | 12 | 1 | 2 3 |
| 1.New and additional excavation section | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Temporary Facities of Tunnnel entrance | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Tunnnel | | | | | | | | | | | | | • [| Exca | avat | ion | | | | | | | | | | | | | | | | | ast | sho | tc re | te | | + | _ | | | _ | \vdash | | | |
| 2.Approach Tunnnel | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | T | | | | | | | | |
| Tunnnel | | | | | | | | | | | | | | | | | | | | | | | | | | | | - | E | xca | vatio | on 🗖 | | | | | | - | Las | st sh | otci | rete | | | | |
| Muck transport shaft | | | | | | | | | | | | | | | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Muck pit | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 3.Belt-conveyor Tunnel | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 4.Water purification room | | | | | | | | | | | | | | | | | | _ | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 5.Tank Cavern | | | Τ | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | - | - | - | | | | | | | |
| | | | | | | | | | | | | | | | | | | | | | | | | | | | | 1 | | | | | | | | | | | | | | | | | | |

Table 6.1 Construction outline schedule chart of HK-Project

~5 years for excavation

More accurate analysis (ongoing study)

- Elastic, static analysis was conduced
 - one calculation for the whole cavern.
 - evaluate the plasticity region based on elastic analysis
 - Mohr-Coulomb's criterion as failure criteria, general (mean) values for Young's modulus
 - design PS anchors, rockbolts, and shotcrete to support the loosened area.
 - elastic limit of the supports themselves not taken into account
- Elasto-plastic, static analysis (ongoing study)
 - step-by-step calculations for each excavation benches.
 - perform calculation even after the stress exceeds the elastic limit.
 - Hoek-Brown's criterion as failure criteria, revised Young's modulus
 - strain softening calculation
 - Designed supports are considered in the calculation
 - elastic limit of the supports also taken into account.



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affordable cable tension and plasticity region depth for B and CH class.

Remarks on the cavity analysis

- Analysis for CM class contamination yet to be done.
 - Results should depend on size and position of the CM area.
- The cavern construction is feasible but final tuning of the shape and size might be necessary when preconstruction survey is conducted.
- Estimation of cost and construction period to be revised near future.

supplements

Table 2.1 Rock mass classification of Central Research Institute of Electric Power

Industry

| Rock | Description |
|------------------------|--|
| class | |
| | The rock mass is very fresh, and the rock forming minerals and grains |
| Δ | undergo neither weathering nor alteration. Joints are extremely tight and |
| 11 | their surfaces ha no visible sign of weathering. Sound by hammer blow is |
| | clear. |
| | The rock mass is solid. There is no opening joint and crack (even of 1 |
| В | mm). But rock forming minerals and grains undergo a little weathering |
| | and alteration in partly. Sound by hammer blow is clear. |
| | The rock mass is relatively solid. The rock forming minerals and grains |
| au | undergo weathering except for quartz. Rock is contaminated by limonite, |
| СН | etc. The cohesion of joints and cracks are slightly decreased. Clay minerals |
| | remain on the separation surface. Sound by hammer blow is a little dim. |
| | The rock mass is somewhat soft. The rock forming minerals and grains |
| | are somewhat softened by weathering, except for quartz. The cohesion of |
| $\mathbf{C}\mathbf{M}$ | joints and cracks is somewhat decreased and rock blocks are separated by |
| | ordinary hammer blow along the joints. Clay materials remain on the |
| | separation surface. Sound by hammer blow is somewhat dim. |
| | The rock mass is soft. The rock forming minerals and grains are softened |
| OT. | by weathering. The cohesion of joints and cracks is decreased and rock |
| CL | blocks are separated by soft hammer blow along the joints. Clay minerals |
| | remain on the separation surface. Sound by hammer blow is dim. |
| | The rock mass is remarkably soft. The rock forming minerals and grains |
| | are softened by weathering. The cohesion of joints and cracks is almost |
| D | absent. The rock mass collapses by light hammer blow. Clay minerals |
| | remain on the separation surface. Sound by hammer blow is remarkably |
| | dim. |
| | |

Note: CRIEPI: Central Research Institute of Electric Power Industry

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Referencs: R.Yoshinaka, S.Sakurai, K.Kikuchi: 岩盤分類とその適用, Journal of the

Japan

Degraded

Fresh, Solid

Summary of rock classification

Table 2.3: Classification of Rock mass in the Tunnels :

| | Rock mass class (%) | | | | | | | | | | | |
|-------------------|---------------------|------|------|------------------------|------|-----|--|--|--|--|--|--|
| | А | В | СН | $\mathbf{C}\mathbf{M}$ | CL | D | | | | | | |
| -300mL Kita-mukae | 0.0 | 51.6 | 43.6 | 3.0 | 1.8 | 0.0 | | | | | | |
| Tunnel | | 95.2 | | | 4.8 | | | | | | | |
| -300mL Survey | 0.0 | 67.9 | 27.7 | 4.0 | 0.4 | 0.0 | | | | | | |
| Borehole | | 95.6 | | | 4.4 | | | | | | | |
| -370mL Moribuden- | 0.0 | 11.4 | 45.4 | 39.8 3.4 0.0 | | | | | | | | |
| mukae Tunnel | | 56.8 | | 43.2 | | | | | | | | |
| -370mL Shin-2ban- | 0.0 | 4.9 | 55.7 | 25.0 | 14.4 | 0.0 | | | | | | |
| mukae | | 60.6 | | 39.4 | | | | | | | | |
| -370mL Survey | 2.4 | 10.5 | 49.2 | 29.7 | 5.7 | 0.2 | | | | | | |
| Borehole No.2 | | 62.1 | | 35.6 | | | | | | | | |
| -370mL Survey | 0.0 | 19.2 | 59.2 | 16.5 | 3.8 | 0.3 | | | | | | |
| Borehole No.3 | | 78.4 | | 20.6 | | | | | | | | |
| -370mL Survey | 6.6 | 20.5 | 36.4 | 22.6 | 7.1 | 3.1 | | | | | | |
| Borehole No.4 | | 63.5 | | | 32.8 | | | | | | | |
| -430mL Kita-mukae | 0.0 | 18.1 | 39.0 | 38.1 | 1.9 | 2.9 | | | | | | |
| Tunnel | | 57.1 | | | 42.9 | | | | | | | |

• Suggesting that the area at 300mL and above has fairly good rock conditions.

No data below
-430mL

Factor of safety

Using the Mohr-Coulomb criterion, the local factor of safety is calculated and compared a series of 3D stress analyses by FLAC3D.



Fig. Factor of safety(Strength/stress ratio) for Mohr-Coulomb failure criterion. (Left : relation of normal and shear stresses, Right : relation of major and minor principal stresses)

The strength for the stress state represented by green circle is determined by holding mean stress(σ_m) while increasing or decreasing normal stresses(σ_1, σ_3) until red circle touches the envelope. The ratio of the radii of the two circles is the factor of safety.

collecting rock quality data for shallower levels.



<主要断層 西北西からの鳥瞰>



茂住鉱岩盤調査

- ・調査坑道の岩盤は比較的堅硬であり、L級以下の岩盤は出現しない。
- ・調査坑道の周辺坑道では断層・亀裂があり、M・L級の岩盤が多い。
- ホーリングNo.1孔の岩盤は調査坑道とほぼ同様の比較的堅硬な岩盤状況であるが 深度60m以深ではL級の岩盤が出現する。
- ・ホーリングNo.2孔では深度50m付近までは調査坑道と同様であるが、50m以深では M・L級を主体とした岩盤となる。
- ・調査坑道入口では13号ヒ断層を確認した。

